

GENERAL NOTES:

- THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE CIVIL DRAWINGS AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES, AND ADDITIONAL ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
- THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2 PROVISIONS OF THE NORTH CAROLINA STATE BUILDING CODE, 2012 EDITION.
- PORTIONS OF THE STRUCTURE NOT ALTERED AND NOT AFFECTED BY THE ALTERATION HAVE NOT BEEN DESIGNED TO COMPLY WITH THE CODE REQUIREMENTS FOR A NEW STRUCTURE.
- BEFORE PROCEEDING WITH WORK WITHIN THE EXISTING STRUCTURE. THE CONTRACTOR SHALL BECOME FAMILIAR WITH THE EXISTING STRUCTURAL CONDITIONS. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE DESIGN AND ERECTION OF ANY AND ALL SAFEGUARDS NECESSARY TO PROTECT THE EXISTING STRUCTURE AT ALL TIMES DURING THE PROCESS OF DEMOLITION AND CONSTRUCTION.
- THE CONTRACTOR SHALL FIELD VERIEV THE DIMENSIONS ELEVATIONS THE CONTRACTOR SHALL FIELD VERIFY THE DIMENSIONS, ELEVATIONS AND OTHER REQUIREMENTS NECESSARY FOR THE PROPER CONSTRUCTION AND ALIGNMENT OF THE NEW PORTIONS OF THE STRUCTURE TO THE EXISTING, ANY DIMENSIONS SHOWN OF EXISTING STRUCTURES SHALL BE CONSIDERED AS APPROXIMATE AND ADEQUATE FOR BIDDING PURPOSES ONLY, THE CONTRACTOR SHALL MAKE ALL MEASUREMENTS NECESSARY FOR THE FARICATION AND ERECTION OF STRUCTURAL MEMBERS. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER
- 6. DESIGN CRITERIA:

LIVE LOADS - UNIFORM: DECKS

DEMOLITION NOTES:

1. SOME TERMS INDICATED ON PLAN ARE DEFINED AS FOLLOWS:

REMOVE: DETACH ITEMS FROM EXISTING CONSTRUCTION AND LEGALLY DISPOSE OF THEM OFF-SITE.

REMOVE AND SALVAGE: DETACH ITEMS FROM EXISTING CONSTRUCTION AND DELIVER THEM TO THE OWNER READY FOR REUSE.

REMOVE AND REINSTALL: DETACH ITEMS FROM EXISTING CONSTRUCTION, PREPARE THEM FOR REUSE, AND REINSTALL THEM WHERE INDICATED.

- EXISTING TO REMAIN: EXISTING ITEMS OF CONSTRUCTION THAT ARE NOT TO BE REMOVED.
- 2. COMPLY WITH LOCAL NOISE, DUST AND EROSION CONTROL REGULATIONS. CONTROL DUST FROM DEMOLITION TO PREVENT IT FROM SPREADING TO OCCUPIED PORTIONS OF BUILDING AND TO AVOID CREATING A NUISANCE IN SURROUNDING AREA
- 3 OBTAIN REQUIRED PERMITS FROM GOVERNING AUTHORITIES
- 4. PROVIDE TEMPORARY BARRICADES AND OTHER PROTECTION REQUIRED O PREVENT INJURY TO PEOPLE AND DAMAGE TO ADJACENT PROPERT
- 5 PROTECT FROM DAMAGE EXISTING ROADS WALKS CURRS LANDSCAPE AND OTHER SITE AND BUILDING STRUCTURES. REPAIR OR REPLACE DAMAGED ITEMS.
- 6 REMOVE MATERIAL RESULTING FROM DEMOLITION OPERATIONS AND DISPOSE OF IN ACCORDANCE WITH APPLICABLE LAWS AND REGULATIONS AS PART OF THE WORK. CONTROL RUBBISH, DEBRIS, AND DUST BY APPROVED METHODS, AS REQUIRED BY LOCAL NOISE, DUST, AND EROSION CONTROL REGULATIONS, SOLID WASTE DISPOSAL, SOLID WASTE MANAGEMENT, DISPOSAL OF SOLID WASTE IN OPEN DUMPS IS PROHIBITED.
- ALL PIPING AND CONDUIT FOR UTILITIES IN THE AREA OF WORK SHALL BE TEMPORARILY MOVED AND SUPPORTED DURING CONSTRUCTION. ONCE DEMOLITION AND NEW FRAMING WORK IS COMPLETE, REINSTALL ALL PIPING TO ITS ORIGINAL POSITION.

HELICAL PIER NOTES:

- 1. HELICAL PIERS SHALL CONTAIN HELICAL LEAD SECTIONS AND SQUARE SHAFT EXTENSIONS AS MANUFACTURED BY THE A B. CHANCE CO. OR CONTRACT ON THE SUPPLIERS MAY BE ACCEPTABLE PENDING APPROPRIATE SUBMITTALS TO THE STRUCTURAL ENGINEER OF RECORD). HELICAL PIERS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS
- 2. HELICAL PIER DESIGN CAPACITIES ARE AS FOLLOWS: A. COMPRESSION..... 10 KIPS
- 3. BASE BID LENGTH SHALL BE 20 FEET FOR ALL HELICAL PIERS.
- 4. HELICAL PIERS SHALL BE INSTALLED TO A MAXIMUM TOLERANCE IN ANY DIRECTION OF 3 INCHES PER PIER. WHERE PIER IS OUT OF POSITION MORE THAN 3 INCHES IN ANY DIRECTION, THE CONTRACTOR MAY BE REQUIRED TO CONSTRUCT OR MODIFY STRUCTURAL ELEMENTS TO COMPENSATE FOR THE ECCENTRICTY OF THE INSTALLED HELICAL PIER. THE COST OF CORRECTIVE WORK, INCLUDING THE COST OF RE-ENGINEERING, SHALL BE PAID BY THE CONTRACTOR.
- CONTRACTOR SHALL EMPLOY A REGISTERED LAND SURVEYOR TO SURVEY 5 CONTROL OF STREED ENT EDV TO STREED BOTTE TO DETERT THE TOP ELEVATION OF THE COLONER, THE LOCATION OF THE CENTER OF EACH PIER, AND THE PLUMENESS OF EACH PIER, TOP ELEVATIONS SHALL BE GIVEN AS DEVIATIONS FROM THE CUTOFF ELEVATION LOCATIONS OF THE PIERS SHALL BE GIVEN AS A DEVIATION FROM THE INDICATED PLAN LOCATION OF THE PIERS IN EACH OF TWO PERPENDICULAR DIRECTIONS, ALL DIMENSIONS AND ELEVATIONS SHALL BE REPORTED TO WITHIN % INCH.

CAST-IN-PLACE CONCRETE NOTES:

- 1. CONCRETE SHALL BE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301, [350], AND 318.
- 2. CONCRETE SHALL BE NORMAL WEIGHT AND SHALL OBTAIN 28 DAY COMPRESSIVE STRENGTHS AS FOLLOWS PILECAPS AND GRADE BEAMS4,000 PSI
- 3. REINFORCING MATERIALS SHALL BE AS FOLLOWS: REINFORCING BARS - ASTM A 615, GRADE 60, DEFORMED.
- 5. ALL REINFORCING BARS AND EMBEDDED ITEMS SUCH AS ANCHOR RODS AND WELD PLATES SHALL BE ACCURATELY PLACED AND ADEQUATELY TIED AND SUPPORTED BEFORE CONCRETE IS PLACED TO PREVENT DISPLACEMENT BEYOND PERMITTED TOLERANCES.
- 6. CONCRETE COVER TO REINFORCING BARS SHALL CONFORM TO THE MINIMUM COVER RECOMMENDATIONS IN ACI 318, UNLESS THE DRAWINGS SHOW GREATER COVER REQUIREMENTS
- 7. LAP CONTINUOUS REINFORCING BARS 57 X BAR DIAMETER, TYPICAL UNI ESS OTHERWISE NOTED

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- ROUGH CARPENTRY SHALL BE IN ACCORDANCE WITH THE AMERICAN 1. FOREST AND PAPER ASSOCIATION (AF&PA) "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
- UNLESS OTHERWISE NOTED, USE "COMMON" NAILS AND ALL NAILING SHALL CONFORM TO THE "FASTENING SCHEDULE" TABLE 2304.9.1 OF THE 2 BUILDING CODE.
- WOOD FRAMING MEMBERS SHALL COMPLY WITH PS 20 "AMERICAN SOFTWOOD LUMBER STANDARD" AND THE FOLLOWING REQUIREMENTS: MOISTURGO LOMBER STANDARD AND THE FOLLOWING REQUIREMENT A. MOISTURE CONTENT: SEASONED, WITH 19 PERCENT MAXIMUM MOISTURE CONTENT.
 GRADE - NO. 2, OR BETTER UNLESS OTHERWISE NOTED.
 SPECIES - SOUTHERN PINE GRADED UNDER SPIB RULES.

- 5. ALL WOOD FRAMING MEMBERS SHALL BE PRESERVATIVE-TREATED.
- ALL CONNECTION HARDWARE IN CONTACT WITH PRESERVATIVE TREATED 6 WOOD SHALL BE HOT DIPPED GALVANIZED COATED

ABBREVIATIONS: ALUMIN

GRADE BEAM

HOOK

ALUM

ALUM ARCH BD BM BOD BRG BTWN C TO C

CL CLR

COL CONC

CONN CONSTR

CONT

CTR CTRD CW DBL DIA DWGS EA EF EL EMBED

EOD

EQ EW

EW EXIST EXP FDN FO FIN FOC FRMG FTG FV

GALV

GR BM

нк

ALUININUM ADDITECT BAR DIAMETER BEAM BOTTOM OF DECK BEARING CENTER TOCENTER CENTER TOCENTER CENTER TOCENTER COLUMN CONCRETE COLUMN CONCRETE CONTINUOUS CONTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONSTRUCTION CONTRUCTS CONTINUOUS CONT	OPNG PAR PC PERP PL REF REINF REQD SCHED SIM SL THK TOC TOF TYP UON	REINFORCING
FACE OF CONCRETE		

PLAN LEGEND:

- SECTION/DETAIL NUMBER/LETTER
- = SECTION/DETAIL MARK
- SHEET NI IMPED WHERE
- SECTION/DETAIL IS DRAWN

CP Y = GRADE BEAM MARK

= FIELD VERIFY

TOWN OF BEAUFORT BEAUFORT BULKHEAD AND BOARDWALK REPAIR BEAUFORT, NORTH CAROUNA 2005 NOTES GENERAL

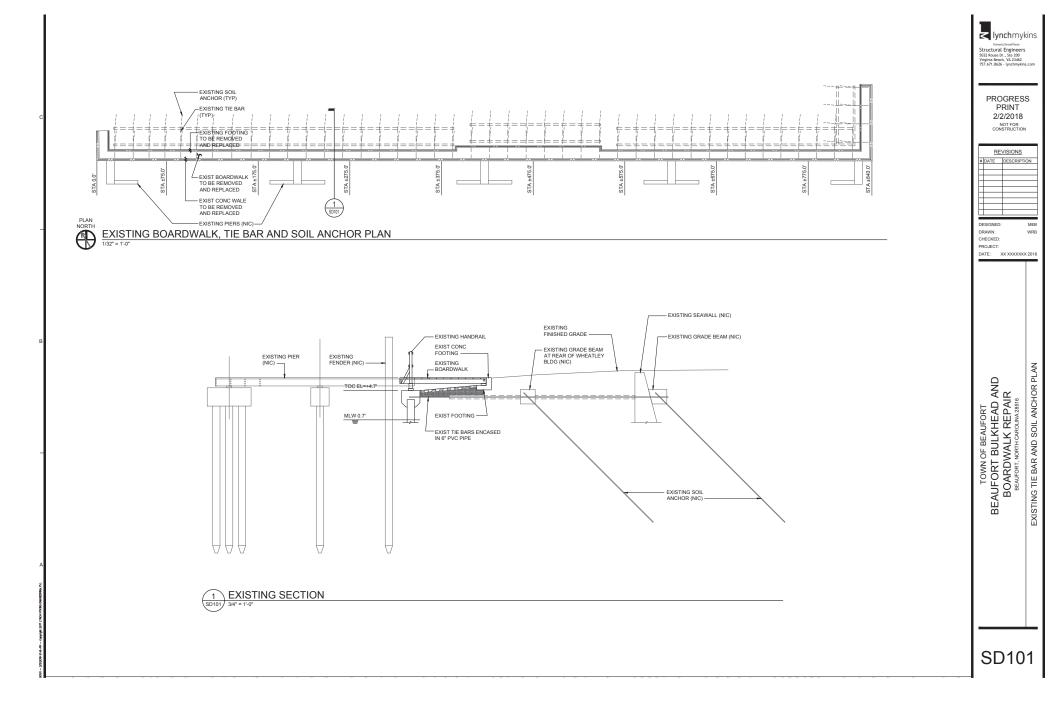
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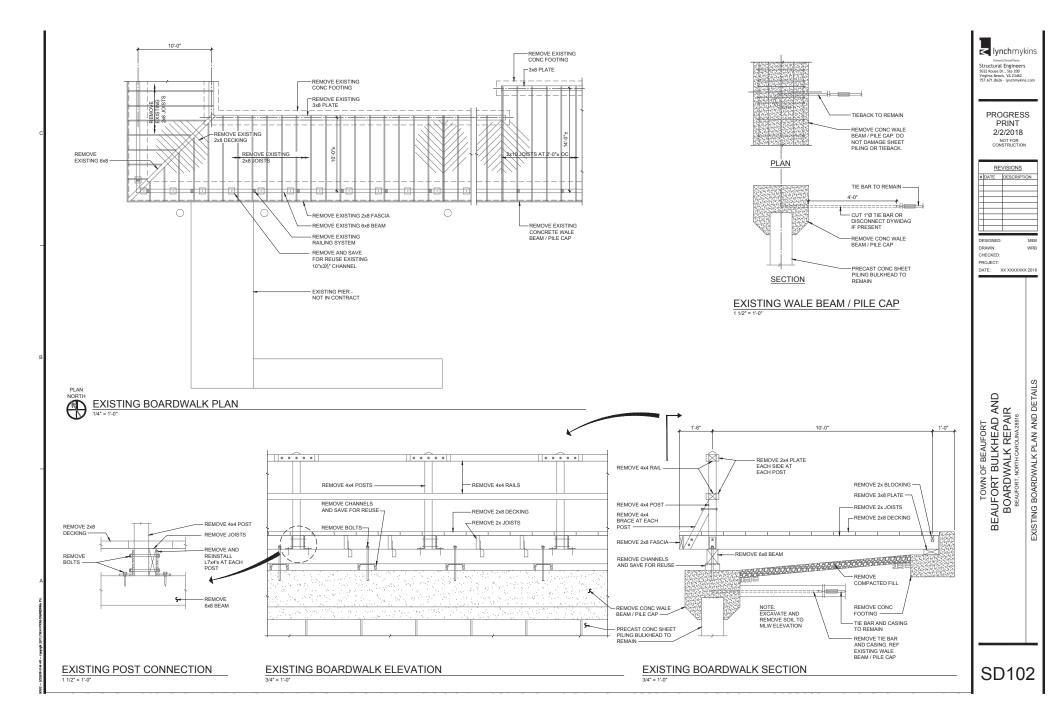
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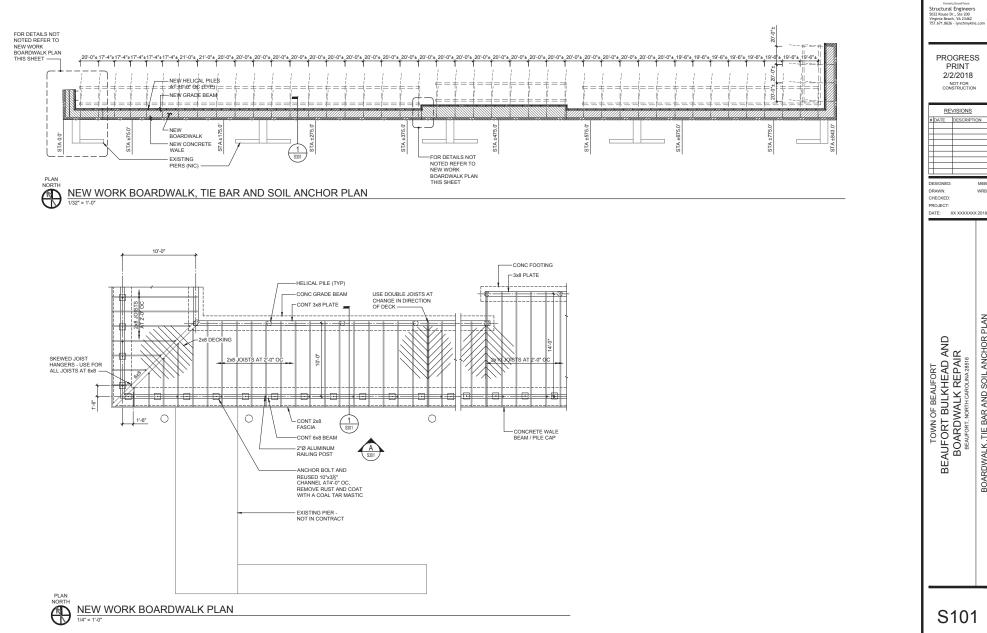
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DESIGNED DRAWN WRE CHECKED PROJECT: DATE: XX XXXXXXX 2018







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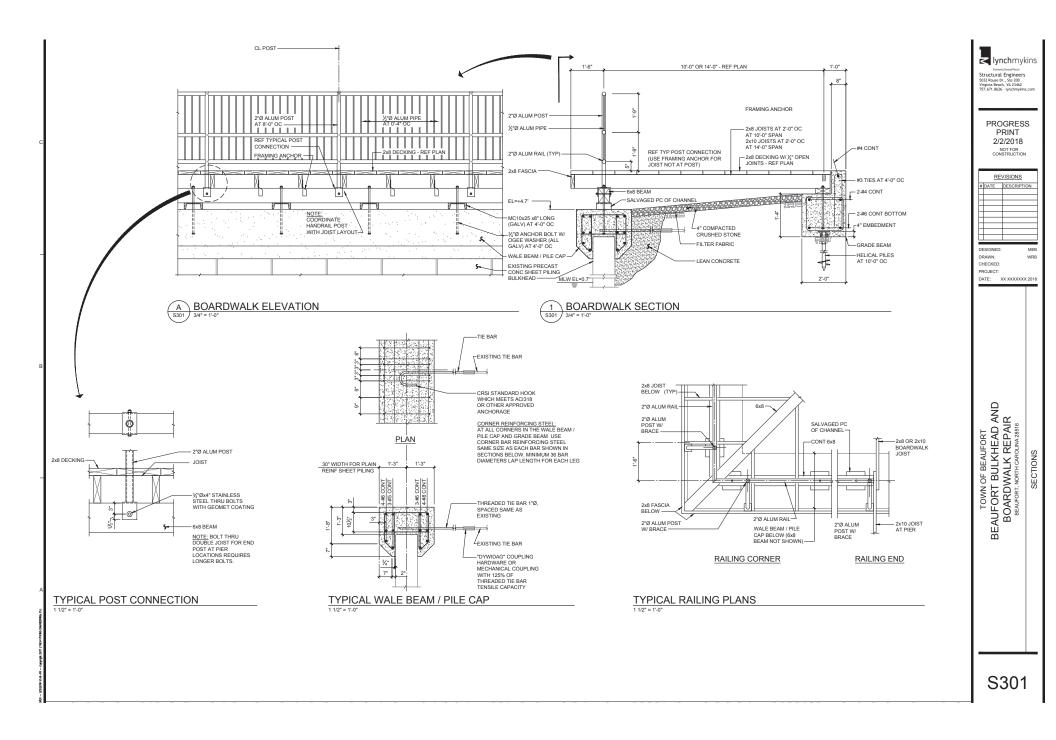
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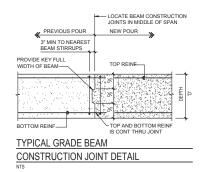
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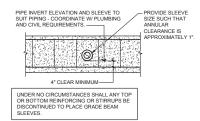
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BOARDWALK, TIE BAR AND SOIL ANCHOR PLAN







TYPICAL SLEEVED GRADE BEAM DETAIL



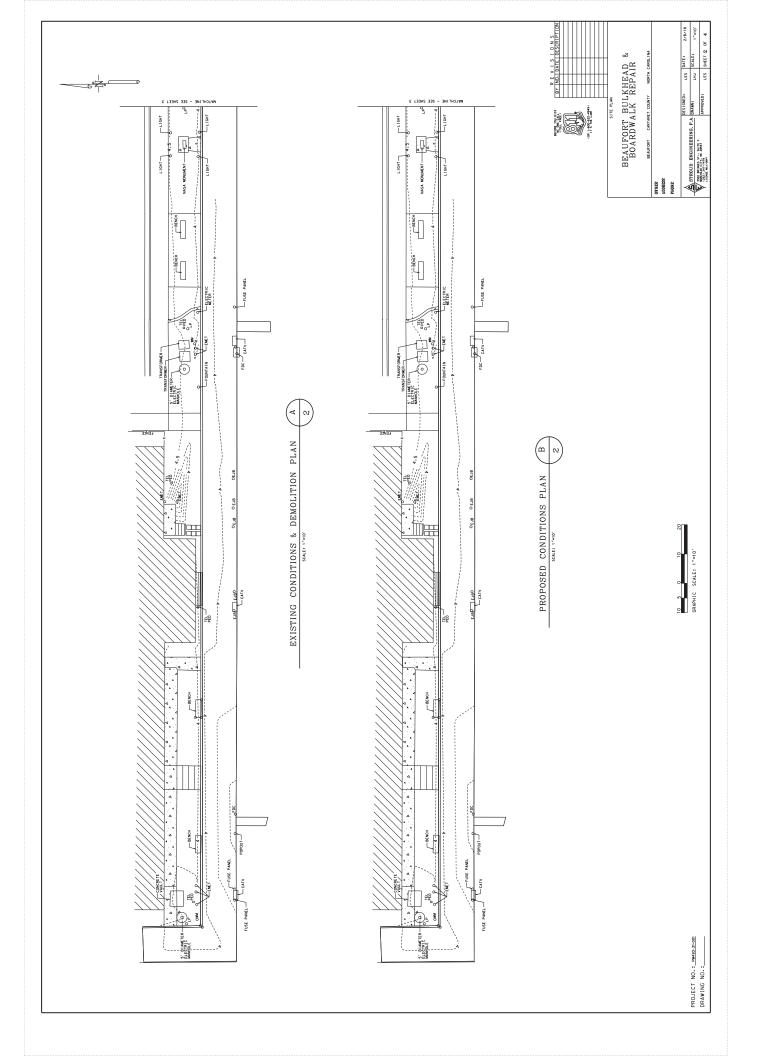
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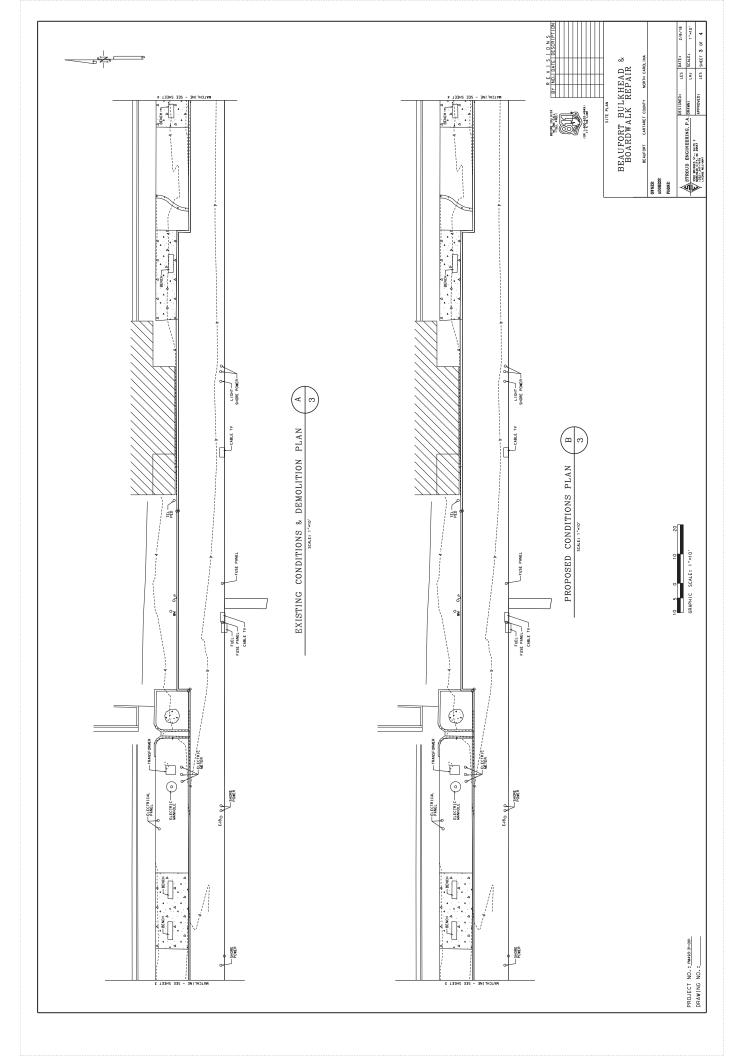
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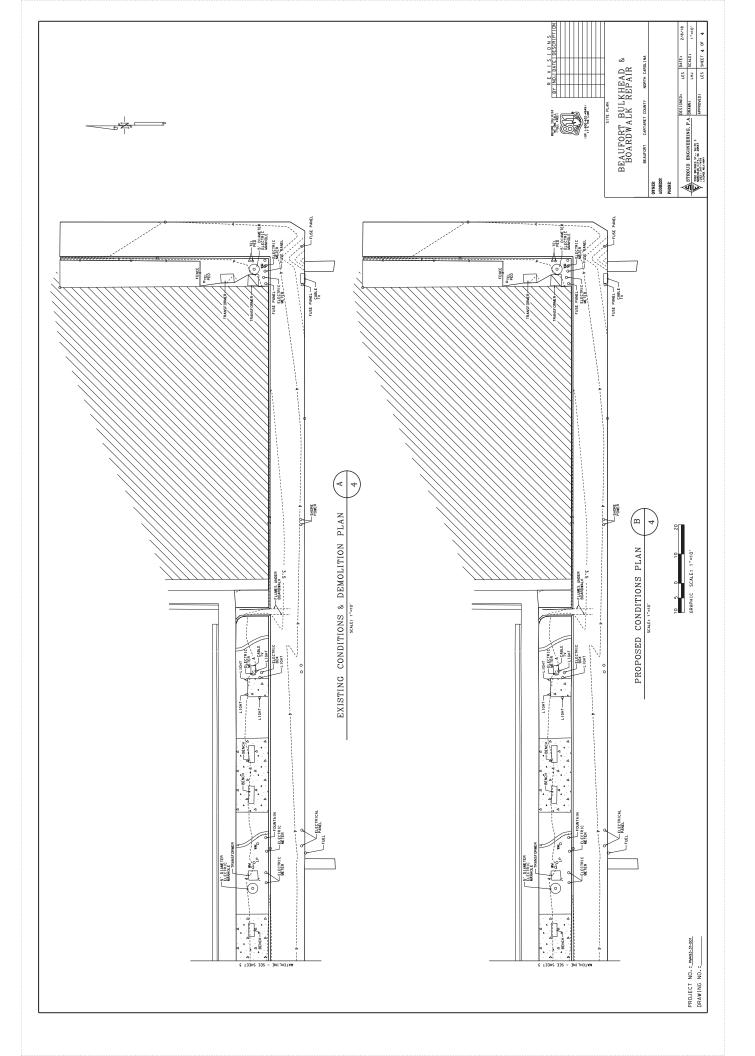
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TOWN OF BEAUFORT BEAUFORT BULKHEAD AND BOARDWALK REPAIR BEAUFORT, NORTH CAROUNA 20216 TYPICAL DETAILS

S501







Project	Beaufort Bulkhead and Boardwalk Repair	LM Project # V17.065
Date	3/6/2018	

Demolition of Existing Boardwalk and Concerte Seawall Pile Cap

Remove Hand Rails	9,000
940 linear feet	
Remove Deck Boards and wood framing	48,000
40,000 board feet	
Remove Concrete spread footing	
940 linear feet	
Remove Pile Cap from Seawall	
Saw cut 5 deep inches each side	
Tempoary construction netting to keep concrete from water	
Break up with jack hammer	
Dispose of Debris	65,000
720 tons Load and Haul to dump (includes dump fee of \$38/ton)	
Errosion and Sediment Control	
Temporary Fencing to Close off site (Assume 6 month Duration)	

Construction of New Boardwalk and Concrete Seawall Pile Cap

Helical Piles		67,200
96 Piles 20 feet long		
Concrete for Pile Cap and Grade Beam		208,200
305 cubic yards concrete		
22 tons of rebar		
Formwork		
Clean seawall for form installation		
Concrete Admixture for waterproofing pile cap		10,800
Velosit CA 115		
Excavate behind seawall and backfill with lean concrete		67,500
425 cubic yards of excvation		
425 cubic yards of lean concrete		
Remove 4' of Tie rods and replace		12,000
46 Tie rods		
Regrade soils below boardwalk		17,200
Install crushed stone layer under boardwalk		39,900
15,000 sf of Filter Fabric		
220 cubic yards of crushed stone		
Rebuild Boardwalk		162,200
11860 SF of boardwalk		
Pressure Treated #2 Southern Pine		
Galvanized Bolts and Nails		
Stainless steel deck screws		
Install Gaurdrails		38,200
940' railing (Wire and pressure treated wood)		
Inspections and Testing		14,800
Mobilization		12,000
	Total	885,300
Unforseen conditions contingency (20%)		177,060
Overhead and profit		159,354



STROUD ENGINEERING, P.A.

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April 10, 2017

Mr. Mark Eakes Town of Beaufort Public Works Director 701 Front Street Beaufort, North Carolina 28516

Re: Preliminary Structural Assessment- Town of Beaufort Front Street Waterfront Plaza

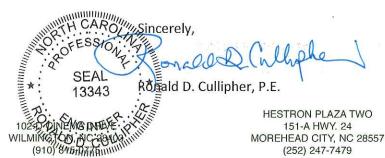
Dear Mr. Eakes,

At your request and in your presence, I made a couple of site visits to the boardwalk area to evaluate the issues identified with the settlement in and around the Dock House. On the second visit, we had Mr. Carl Bonner, P.E., Terracon, a geotechnical firm with a branch located in Greenville, with us as well. Without knowing the original proposed construction details, it was difficult to develop a theory on what was happening. Logic was not making sense with the observations we were seeing. With assistance of several people we were able to ascertain that the original design engineering firm was Rivers and Associates, also from Greenville. Mr. Bonner was able to locate those drawings from Rivers and Associates and we have provided a copy of those drawings to you and have saved a set here as well.

The old drawings have a detail that answers the questions to what is happening. The landward support for the deck joists is supported by a concrete footing without vertical support, (we had been led to believe that this was actually a retaining wall) as time passes on, siltation of the underlying material weeps through the bulkhead and allows settlement of this footing. A permanent repair solution would be to replace this footing with a pile supported footing so that in the future should future loss of material occur the footing would not settle and be structurally independent. This solution would not be cheap. Band aid solutions would be to place soil between the outer wall and this footing and provide some type of weep protection thru the seawall, replace the footing at the proper elevation and then blend the sidewalks back into the boardwalk.

Additionally, during our investigation efforts, significant concrete spalling and corrosive rebar was evident on the seawall and those structural repairs need to be initiated soon as well.

Please review and advise how you would like us to move forward. If you have any questions, please let me know.



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